

# Check Dam (CD)

# **Practice Description**

A check dam is a small barrier or dam constructed across a swale, drainage ditch or other area of concentrated flow for the purpose of reducing channel erosion. Channel erosion is reduced because check dams flatten the gradient of the flow channel and slow the velocity of channel flow. Most check dams are constructed of rock, but hay bales, logs and other materials may be acceptable. Contrary to popular opinion, most check dams trap an insignificant volume of sediment.

This practice applies in small open channels and drainageways, including temporary and permanent swales. It is not to be used in a live stream. Situations of use include areas in need of protection during establishment of grass and areas that cannot receive a temporary or permanent non-erodible lining for an extended period of time.

## **Planning Considerations**

Check dams are used in concentrated flow areas to provide temporary channel stabilization during the intense runoff periods associated with construction disturbances. Check dams may be constructed of rock, logs, hay bales or other suitable material, including manufactured products. MDOT Drawing ECD-4 at the end of this practice shows the typical application of check dam structures. Most check dams are constructed of rock. Rock may not be acceptable in some installations because of aesthetics; therefore, alternative types of check dams need to be considered.

#### **Rock check dams**

Rock check dams (Figures CD-1 and CD-2) are usually installed with backhoes or other suitable equipment, but hand labor is likely needed to complete most installations to the quality needed. The rock is usually purchased, and some locations in the state may not have rock readily available. The use of rock should be considered carefully in areas to be

mowed. Some rock may be washed away during heavy rain events and should be removed before each mowing operation. Additional installation drawings are provided at the end of this practice as MDOT Drawings ECD-8 and ECD-9.

#### Log check dams

Log check dams (Figure CD-3) are more economical from a materials cost standpoint since logs can usually be salvaged from clearing operations. The time and labor required would be greater for log check dams. Increased labor costs would offset the reduced material costs. Log check dams would not be permanent but may last long enough to get grass linings established.

#### Hay bale check dams

Check dams constructed of hay bales (Figure CD-4) have the shortest life of the materials discussed and are only used as a temporary means to help establish a channel to vegetation. MDOT Drawing ECD-5 is provided at the end of this practice and shows more specifics for hay bale check dams. MDOT Drawing ECD-6 shows typical details for a straw wattle ditch check as an alternative to hay bale check dams. Hay bale check dams should not be used where permanent watercourse protection is needed and should be used only in concentrated-flow areas where only minimal runoff occurs.

#### Without proper installation, which is rarely done, hay bale check dams always fail.



Figure CD-1 Profile of Typical Rock Check Dams

Check dams should be planned to be compatible with the other features such as streets, walks, trails, sediment basins and rights-of-way or property lines. Check dams are normally constructed in series, and the dams should be located at a normal interval from other grade controls such as culverts or sediment basins.



Figure CD-2 Cross Section of Typical Rock Check Dam





Figure CD-4 Typical Hay Bale Check Dam (NOTE: Without proper installation, which is rarely done, hay bale check dams always fail.)

# **Design Criteria and Installation**

Formal design is not required. The following limiting factors should be adhered to when designing check dams.

#### **Drainage Area**

Ten acres or less (rock or logs).

## **Maximum Height**

Two feet when drainage area is less than 5 acres.

Three feet when drainage area is 5 to 10 acres.

#### **Depth of Flow**

Six inches when drainage area is less than 5 acres.

Twelve inches when drainage area is 5 to 10 acres.

The top of dam, perpendicular to flow, should be parabolic. The center of the dam should be constructed lower than the ends. The elevation of the center of the dam should be lower than the ends by the depth of flow listed above.

#### **Side Slopes**

2:1 or flatter.

#### Spacing

Elevation of the toe of the upstream dam is at or below elevation of the crest of the downstream dam.

#### Keyway

The rock or log check dam should be keyed into the channel bottom and abutments to a depth of 12 to 24''. The keyway width should be at least 12''. The keyway is to prevent erosion around the end of and beneath the dam. Hay bale check dams should be embedded into the soil at least 3''.

## Rock Check Dams

Rock check dams should be constructed of durable rock riprap. Rock material diameter should be 2'' to 15''.

In soils where failure by piping of soils into the rock is likely, a geotextile will be used as a filter to separate the soils from the rock. Geotextile should conform to the requirements of type I geotextile in Table CD-1.

Property	Test method	Class I	Class II	Class III	Class IV <sup>1</sup>
Tensile strength (lb) <sup>2</sup>	ASTMD 4632 grabtest	180 minimum	120 minimum	90 minimum	115 minimum
Elongation at failure (%) <sup>2</sup>	ASTM D 4632	≥ 50	≥ 50	≥ 50	≥ 50
Puncture (pounds)	ASTM D 4833	80 minimum	60 minimum	40 minimum	40 minimum
Ultraviolet light (% residual tensile strength)	ASTM D 4355 150-hr exposure	70 minimum	70 minimum	70 minimum	70 minimum
Apparent opening size (AOS)	ASTMD 4751	As specified max. no.40 <sup>3</sup>			
Permittivity sec-1	ASTM D 4491	0.70 minimum	0.70 minimum	0.70 minimum	0.10 minimum

 Table CD-1
 Requirements for Nonwoven Geotextile

Table copied from NRCS Material Specification 592.

1 Heat-bonded or resin-bonded geotextile may be used for Classes III and IV. They are particularly well suited to Class IV. Needle-punched geotextile is required for all other classes.

2 Minimum average roll value (weakest principal direction).

3 U.S. standard sieve size.

#### **Site Preparation**

Determine location of any underground utilities.

Locate and mark the site for each check dam in strategic locations (to avoid utilities and optimize effectiveness of each structure in flattening channel grade).

Remove debris and other unsuitable material that would interfere with proper placement of the check dam materials.

Excavate a shallow keyway (12''-24'' deep and at least 12'' wide) across the channel and into each abutment for each check dam.

#### **Materials Installation**

As specified, install a non-woven geotextile fabric in the keyway in sandy or silty soils. This may not be required in clayey soils.

Construct the dam with a minimum 2:1 side slope over the keyway and securely embed the dam into the channel banks. Position rock to form a parabolic top, perpendicular to channel flow, with the center portion at the elevation shown in the design so that the flow goes over the structure and not around the structure.

#### **Erosion and Sediment Control**

Install vegetation (temporary or permanent seeding) or mulching to stabilize other areas disturbed during the construction activities.

#### **Construction Verification**

Check finished size, grade and shape for compliance with standard drawings and materials list (check for compliance with specifications if included in contract specifications).

## **Common Problems**

Consult with a qualified design professional if any of the following occur:

Variations in topography on site indicate check dam will not function as intended. Change in plan will be needed.

Materials specified in the plan are not available.

# Maintenance

Inspect the check dam for rock displacement and abutments for erosion around the ends of the dam after each significant rainfall event. If the rock appears too small, add additional stone and use a larger size.

Inspect the channel after each significant rainfall event. If channel erosion exceeds expectations, consult with the design professional and consider adding another check dam to reduce channel flow grade.

Sediment should be removed if it reaches a depth of <sup>1</sup>/<sub>2</sub> the original dam height. If the area behind the dam fills with sediment, there is a greater likelihood that water will flow around the end of the check dam and cause the practice to fail.

Check dams may be removed when their useful life has been completed. The area where check dams are removed should be seeded and mulched immediately unless a different treatment is prescribed. In some instances check dams should be left as a permanent measure to support channel stability.

## References

### **BMPs from Volume 1**

Chapter 4 Temporary Seeding (TS)	4-103
MDOT Drawing ECD-4	
Ditch Check Structures, Typical Applications and Details	4-124
MDOT Drawing ECD-5	
Temporary Erosion, Sediment and Water Pollution Control Measures, Silt Fence and Hay Bale Ditch Check	4-125
MDOT Drawing ECD-6	
Details of Erosion Control Wattle Ditch Check	4-126
MDOT Drawing ECD-7	
Details of Erosion Control Silt Dike Ditch Check	4-127
MDOT Drawing ECD-8	
Rock Ditch Check	4-128
MDOT Drawing ECD-9	
Rock Ditch Check with Sump Excavation	4-129
MDOT Drawing ECD-20	
Details of Erosion Control Sandbag Ditch Check	4-130













